

FED. ROAD DIST. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	RI	BRO-0784(002)	2015	1	32

INDEX

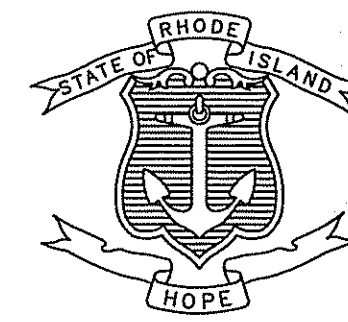
BRIDGE PLANS

SHEET NO.	DESCRIPTION	STAMP
1	COVER SHEET	VHB
2	BRIDGE DRAWING ABBREVIATIONS	VHB
3	JOB SPECIFIC GENERAL NOTES 1	VHB
4	JOB SPECIFIC GENERAL NOTES 2	VHB
5	JOB SPECIFIC GENERAL NOTES 3	VHB
6	JOB SPECIFIC GENERAL NOTES 4	VHB
7	BRIDGE GENERAL PLAN AND LONGITUDINAL SECTION	GZA
8	TYPICAL TRANSVERSE SECTION	VHB
9	BRIDGE PROFILE	VHB
10	EXISTING BRIDGE PLAN AND LONGITUDINAL SECTION	GZA
11	BRIDGE DEMOLITION PLAN & SECTION	GZA
12	BRIDGE EXCAVATION PLAN AND SECTION	GZA
13	NORTH ABUTMENT SECTIONS AND DETAILS	GZA
14	SOUTH ABUTMENT SECTIONS AND DETAILS	GZA
15	ABUTMENT DETAILS	GZA
16	SCOUR PROTECTION PLAN	GZA
17	SLOPE PROTECTION DETAILS	GZA
18	PRECAST BEAM SEAT DETAILS	VHB
19	FRAMING PLAN	VHB
20	STEEL DETAILS 1	VHB
21	STEEL DETAILS 2	VHB
22	DECK DETAILS 1	VHB
23	RAILING DETAILS 1	VHB
24	RAILING DETAILS 2	VHB

HIGHWAY PLANS

SHEET NO.	DESCRIPTION	STAMP
25	STANDARD PLAN SYMBOLS & STANDARD LEGEND	VHB
26	STANDARD NOTES - 1	VHB
27	STANDARD NOTES - 2	VHB
28	TYPICAL SECTION AND JOB SPECIFIC LEGEND	VHB
29	GENERAL PLAN	VHB
30	LOCATION PLAN	VHB
31	CONSTRUCTION STAGING PLAN	VHB
32	TRAFFIC CONTROL PLAN	VHB

STATE OF RHODE ISLAND



DEPARTMENT OF TRANSPORTATION

DESIGN / BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE AND APPROACHES

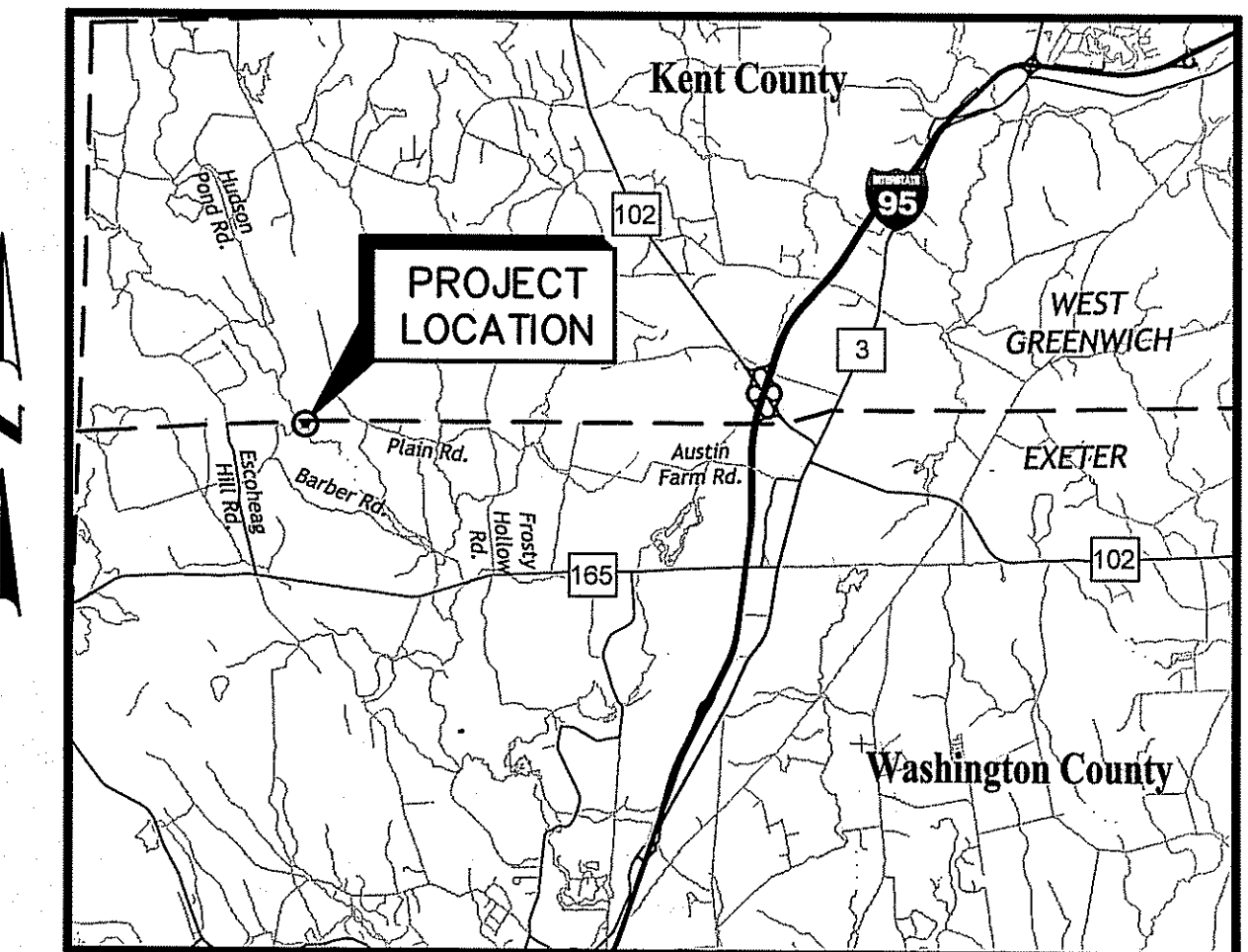
BRIDGE NO. 128401 (EXISTING 784)

EXETER

WASHINGTON COUNTY

R.I. CONTRACT NO. 2011-DB-100 F.A. PROJECT NO. BRO-0784(002)

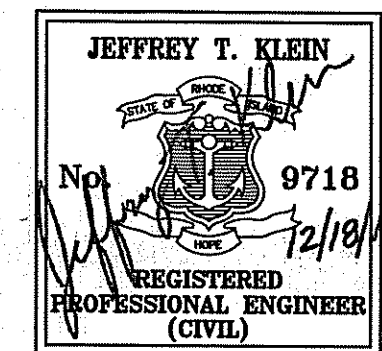
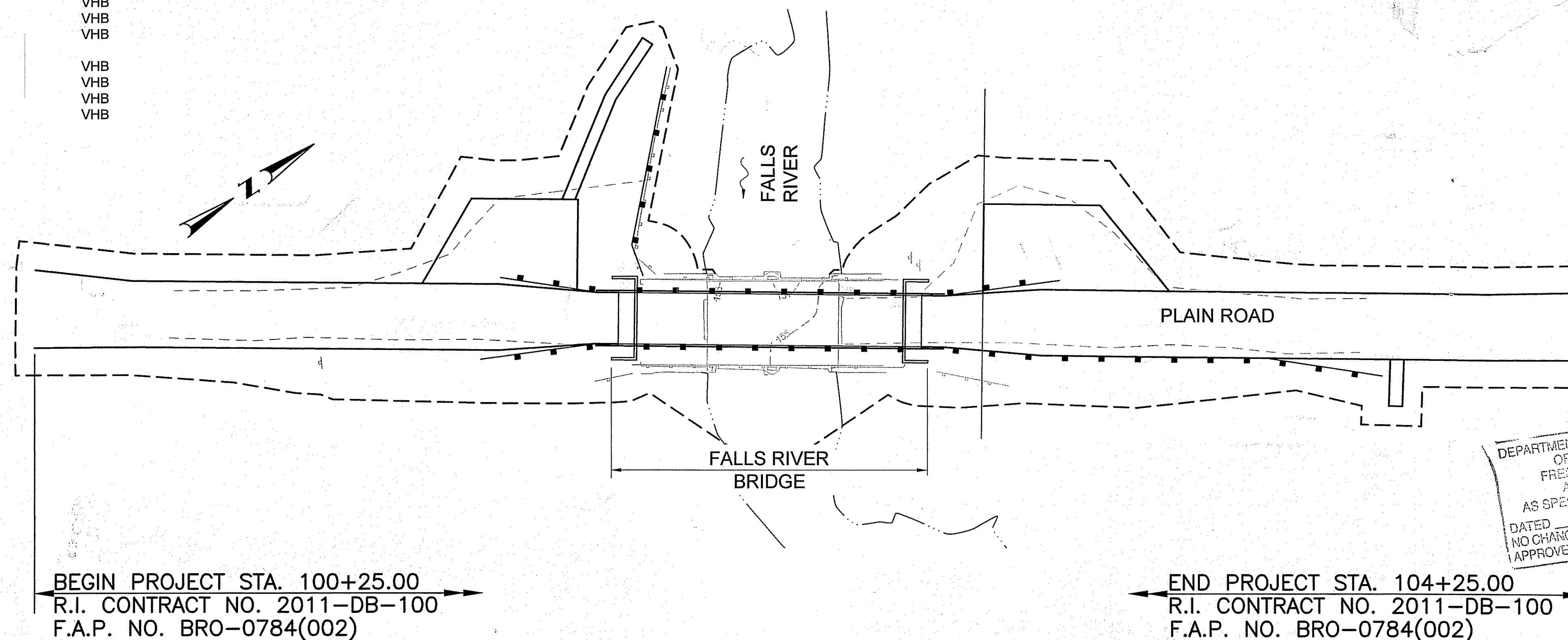
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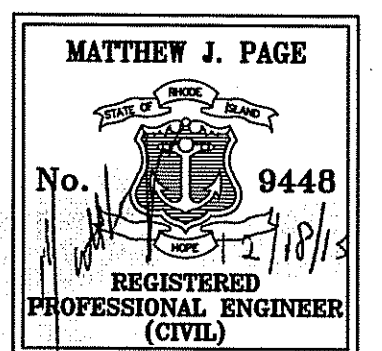
LOCATION MAP
SCALE: 1"=10000'

TRAFFIC DESIGN DATA

2015 AADT	20 V.P.D.
2035 AADT	25 V.P.D.
D	50%
K	25%
T	2%
2013 DHV	5 V.P.H.
2033 DHV	6 V.P.H.
DESIGN SPEED	25 M.P.H.



VHB PLANS



GZA PLANS

PERMIT SUBMISSION
DECEMBER 2015

DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
OFFICE OF WATER RESOURCES
FRESHWATER WETLANDS PROGRAM
APPROVED WITH CONDITIONS
AS SPECIFIED IN THE LETTER OF APPROVAL
DATE: FEB 12 2016 FILE # 16-0001
NO CHANGES ALLOWED WITHOUT PROJECT APPROVAL
APPROVED PLANS MUST BE AT CONSTRUCTION SITE

DEPUTY CHIEF ENGINEER	DATE
APPROVED	JAN 8 2016
CHIEF ENGINEER	DATE
APPROVED	
DIRECTOR	DATE

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
APPROVED
DIVISION ADMINISTRATOR DATE

Contract Number 2011-DB-100
Number of Sheet 1
Total Sheets 31

R.I. STANDARD SPECIFICATIONS AND STANDARD DETAILS
SPECIFICATIONS TO GOVERN THIS PROJECT ARE THE R.I. STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AMENDED 2010, AND THE STATE AND FEDERAL SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS. STANDARD DETAILS FOR THIS PROJECT ARE R.I. STANDARD DETAILS, 1998 EDITION, WITH ALL REVISIONS.

PLAN
SCALE: 1"=20'-0"
SCALES OF DRAWINGS
AS SHOWN

BASE OF LEVELS
VERTICAL DATUM NAVD 88
HORIZONTAL DATUM NAD 83 (2011) (EPOCH: 2010.0000)



FED. ROAD DIV. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
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GENERAL NOTES

- ALL CONSTRUCTION INDICATED ON THESE PLANS SHALL BE IN ACCORDANCE WITH:
 - * THE STATE OF RHODE ISLAND STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AMENDED 2010 INCLUDING ALL REVISIONS OR SUPPLEMENTS UP TO YEAR 2015.
 - * THE 2007 EDITION OF THE STATE OF RHODE ISLAND DEPARTMENT OF TRANSPORTATION LRFD BRIDGE DESIGN MANUAL, INCLUDING ALL REVISIONS OR SUPPLEMENTS UP TO YEAR 2015.
 - * AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS, 3RD EDITION, INCLUDING LATEST INTERIMS. THE SPECIFICATIONS ACCOMPANYING THESE PLANS.

IN CASE OF CONFLICT BETWEEN THE PLANS, SPECIFICATIONS OR MANUAL LISTED ABOVE, THE SPECIAL PROVISIONS OF THE SPECIFICATIONS ACCOMPANYING THESE PLANS SHALL GOVERN.
- DIMENSIONS, STATIONS, AND ELEVATIONS ARE SHOWN TO THE NEAREST ONE-HUNDREDTH OF A FOOT OR ONE-EIGHTH OF AN INCH, EXCEPT STRUCTURAL STEEL DIMENSIONS WHICH ARE TO THE NEAREST ONE-SIXTEENTH OF AN INCH.
- ALL ELEVATIONS ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).
- ANGLES ARE SHOWN TO THE NEAREST SECOND.
- ALL ABUTMENTS AND WALLS ARE DRAWN LOOKING AT THE EXPOSED FACES.
- ALL ITEMS NOT REFERENCED FOR MODIFICATION WILL BE "EXISTING TO REMAIN" UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- ANY STRUCTURAL MEMBERS THAT ARE TO REMAIN IN USE BUT ARE DAMAGED DURING DEMOLITION SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE, TO THE ENGINEER'S SATISFACTION.
- ANY DAMAGE TO EXISTING STATE OR PRIVATE PROPERTY CAUSED BY THE CONTRACTOR'S OPERATIONS SHALL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST THE STATE.
- THE CONTRACTOR SHALL PLACE ALL EQUIPMENT AND MATERIAL IN HIS FIELD YARD OR AT THE APPROVED LOCATIONS SHOWN ON THE CONSTRUCTION STAGING PLAN. ANY REQUEST TO USE AN ALTERNATIVE LOCATION MAY BE SUBJECTED TO APPROVAL BY RIDEM. SUCH APPROVAL SHALL BE OBTAINED AT THE EXPENSE OF THE CONTRACTOR THROUGH COORDINATION WITH THE RIDOT NATURAL RESOURCES UNIT PRIOR TO APPROVAL BY THE RIDOT ENGINEER. THE EQUIPMENT AND MATERIAL SHALL BE PLACED IN A STORAGE AREA SO AS NOT TO CAUSE A SAFETY HAZARD.
- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL ELEVATIONS, DIMENSIONS, DETAILS, ANGLES, STRUCTURAL MEMBER SIZES, AND LAYOUTS AS SHOWN ON THESE PLANS. THIS PRIOR FIELD VERIFICATION IS ESPECIALLY PERTINENT FOR PRE-FABRICATED STRUCTURAL ITEMS AND WORK IN THE VICINITY OF UTILITIES.
- THE CONTRACTOR SHALL TAKE THE PROPER PRECAUTION TO ENSURE THE STABILITY OF ALL STRUCTURAL ELEMENTS DURING ALL PHASES OF CONSTRUCTION UNTIL THE TOTAL STRUCTURE IS IN PLACE.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COORDINATION WITH UTILITY COMPANIES.
- COORDINATES USED ON THESE PLANS ARE BASED ON THE STATEWIDE COORDINATE SYSTEM, THE NORTH AMERICAN DATUM OF 1983 (NAD 83).
- TOPOGRAPHIC CONDITIONS WERE OBTAINED FROM AERIAL PHOTOGRAMMETRY. ACCURACY OF VERTICAL TOPOGRAPHY IS WITHIN 10% OF ONE-HALF THE CONTOUR INTERVAL.
- FOR BENCH MARKS AND TIES SEE HIGHWAY LOCATION PLANS.
- ALL WORKING POINTS ARE SHOWN AT THE CENTERLINES OF ABUTMENT BEARINGS, UNLESS OTHERWISE NOTED.
- CONTRACTOR SHALL SECURE ALL WORK AREAS AT ALL TIMES TO PREVENT UNAUTHORIZED ACCESS.
- NO IN-WATER WORK IS ALLOWED BETWEEN MARCH 20 AND JUNE 20 AND/OR BETWEEN SEPTEMBER 20 AND DECEMBER 20 OF ANY CALENDAR YEAR. ANY IN-WATER WORK PROPOSED PRIOR TO JULY 1 IN ANY CALENDAR YEAR SHALL REQUIRE WRITTEN APPROVAL FROM ACOE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN ALL REQUIRED PERMITS AND APPROVALS, AT NO COST TO THE STATE, AND THROUGH COORDINATION WITH THE RIDOT NATURAL RESOURCES UNIT.
- THE BRIDGE IS LOCATED WITHIN A ZONE A FLOODPLAIN AS SHOWN ON FEMA FIRM PANEL FM44009C0055H EFFECTIVE DATE OCTOBER 19, 2010. ZONE A FLOODPLAINS ARE AREAS SUBJECT TO THE 1 PERCENT ANNUAL CHANCE FLOOD WITH NO BASE FLOOD ELEVATION DETERMINED.
- DEWATERING BASIN LOCATIONS ARE SHOWN ON THE CONSTRUCTION STAGING PLAN. ANY REQUEST TO USE AN ALTERNATIVE LOCATION MAY BE SUBJECT TO APPROVAL BY RIDEM.
- CONTRACTOR SHALL CALL DIG SAFE AT LEAST 72 HOURS PRIOR TO STARTING THE WORK TO VERIFY LOCATIONS OF EXISTING UTILITIES.

DESIGN DATA

DESIGN SPECIFICATIONS:

- * THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 7TH EDITION, 2014.
- * THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL, 2007, INCLUDING ALL REVISIONS OR SUPPLEMENTS UP TO YEAR 2015.
- * ALL OTHER APPLICABLE DESIGN SPECIFICATIONS ARE REFERENCED IN SECTION 1 OF THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL DATED 2010.
- * THE STATE OF RHODE ISLAND STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AMENDED 2010 INCLUDING ALL REVISIONS OR SUPPLEMENTS UP TO YEAR 2015.
- * IN CASE OF CONFLICT, THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL SHALL GOVERN.

LOAD MODIFIERS:

THE LOAD MODIFIERS FOR THIS PROJECT ARE AS FOLLOWS:

- * THE LOAD MODIFIER FOR DUCTILITY SHALL BE TAKEN AS 1.0 FOR ALL LIMIT STATES.
- * THE LOAD MODIFIER FOR REDUNDANCY SHALL BE TAKEN AS 1.0
- * THE LOAD MODIFIER FOR OPERATIONAL IMPORTANCE SHALL BE TAKEN AS 1.0

LOAD FACTORS:

ALL LOAD FACTORS SHALL BE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, EXCEPT AS MODIFIED IN THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL

- * THE LOAD FACTOR FOR LIVE LOAD FOR THE EXTREME EVENT I SHALL BE TAKEN AS ZERO.
- * THE LOAD FACTOR FOR DEAD LOAD FOR THE EXTREME EVENT I AND EXTREME EVENT II SHALL BE TAKEN AS 1.0.
- * THE LOAD FACTOR FOR SETTLEMENT FOR ALL LIMIT STATES SHALL BE TAKEN AS 1.0

LIVE LOADS:

- * THE DESIGN VEHICULAR LIVE LOAD SHALL BE THE HL-93 DESIGNATION ADJUSTED FOR THE DYNAMIC LOAD ALLOWANCE AND MULTIPLE PRESENCE FACTOR.
- * RHODE ISLAND LEGAL LOADS.

WIND LOADING DESIGN DATA:

THE WIND LOADING DESIGN SHALL BE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL, AND AS MODIFIED HEREIN.

- * EXCEPT DURING CONSTRUCTION, THE DESIGN WIND PRESSURE IS BASED ON A DESIGN WIND SPEED OF 120 MPH.
- * THE DESIGN WIND PRESSURES DURING CONSTRUCTION SHALL BE AS SPECIFIED UNDER THE NOTES TITLED "GENERAL NOTES REGARDING TEMPORARY CONSTRUCTION CONDITIONS".

TRAFFIC DATA:

SEE COVER SHEET

THERMAL DESIGN FORCE DATA:

UNIFORM TEMPERATURE EFFECTS HAVE BEEN TAKEN INTO CONSIDERATION IN ACCORDANCE WITH THE PROCEDURE B OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THE MINIMUM DESIGN TEMPERATURE SHALL BE -10 DEGREES F, AND THE MAXIMUM TEMPERATURE SHALL BE 105 DEGREES F.

SEISMIC DESIGN DATA:

- * UNDER 2009 AASHTO LRFD INTERIM, THE SEISMIC CLASSIFICATION IS SEISMIC ZONE 1.

HYDRAULIC DATA

DRAINAGE AREA - SQ. MI. : 17.12

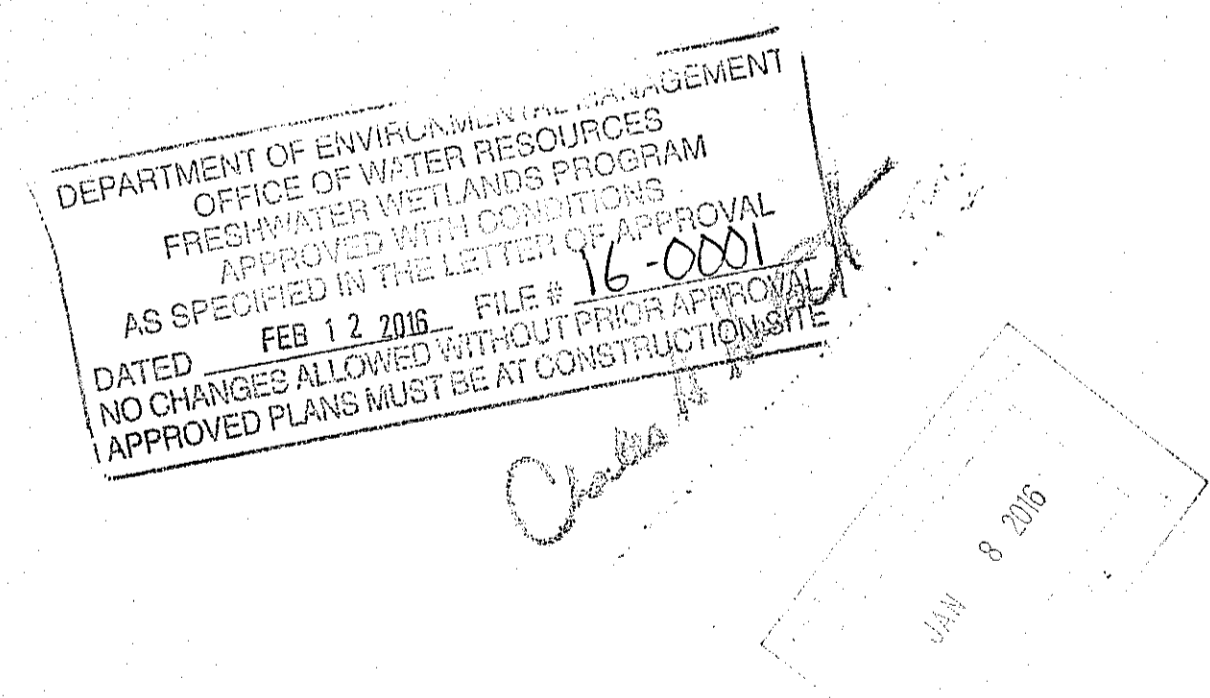
BASIC FLOOD DATA

DISCHARGE Q (100 YEAR) - CFS: 1,300
HIGH WATER ELEVATION - FT: 161.4 (NAVD 88)

FOUNDATION DESIGN DATA:

GRS-IBS BRIDGE ABUTMENTS:

BRIDGE ABUTMENTS DESIGNED IN ACCORDANCE WITH FEDERAL HIGHWAY ADMINISTRATION GEOSYNTHETIC REINFORCED SOIL-INTEGRATED BRIDGE SYSTEM (GRS-IBS) IMPLEMENTATION GUIDE, JANUARY 2011 AND THE RHODE ISLAND DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION, 2004.



REVISIONS			RHODE ISLAND DEPARTMENT OF TRANSPORTATION
NO.	DATE	BY	
			DESIGN/BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE EXETER, RHODE ISLAND
			JOB SPECIFIC GENERAL NOTES 1
			CHECKED BY _____ DATE 11/4/15 SCALE NO SCALE

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STRUCTURAL STEEL NOTES

- FRAMING DIMENSIONS ARE GIVEN ALONG CENTERLINES OF GIRDERS AND ALONG CENTERLINES OF BEARINGS ON ABUTMENTS. THE FABRICATOR IS RESPONSIBLE FOR INCORPORATING THE CAMBER, CROSS SLOPE, AND OTHER EFFECTS THAT MAY IMPACT THE OVERALL GIRDER LENGTHS, DIMENSIONS AND/OR THE DETAILING.
- THE SHOPS FABRICATING THE STRUCTURAL STEEL MUST BE CERTIFIED FOR "MAJOR STEEL BRIDGES (CBR)" IN ACCORDANCE WITH THE AISC QUALITY CERTIFICATION PROGRAM OR EQUIVALENT. SHOPS FABRICATING THE RAILING PLATES SHALL, AT A MINIMUM, BE CERTIFIED FOR "SIMPLE STEEL BRIDGE STRUCTURES (SBR)".

THE SHOPS SHALL ALSO BE CERTIFIED UNDER THE AISC "SOPHISTICATED PAINT ENDORSEMENT (SPE)" QUALITY PROGRAM OR THE SSPC-QP3 PAINT CERTIFICATION PROGRAM.

THE FABRICATOR MUST SUBMIT PROOF OF CURRENT CERTIFICATION AS SPECIFIED.
- THE STEEL ERECTOR/CONTRACTOR FOR THIS PROJECT SHALL BE CERTIFIED FOR "ADVANCED CERTIFIED STEEL ERECTOR (ASCE)" IN ACCORDANCE WITH THE AISC QUALITY CERTIFICATION PROGRAM. THE ERECTOR/CONTRACTOR OF THE STRUCTURAL STEEL SHALL BE REQUIRED TO SUBMIT PROOF OF CURRENT CERTIFICATION AS SPECIFIED, INCLUDING THE QUALITY CONTROL PLAN AND SAFETY PLAN THAT IS REQUIRED TO OBTAIN THE CERTIFICATION.
- FRACTURE CRITICAL MEMBERS (FCM) IF ANY, HAVE BEEN DESIGNATED ON THE STEEL FRAMING AND/OR THE STEEL DETAIL SHEETS.
- SHOP DRAWINGS FOR ALL FABRICATED STEEL INCLUDING RAILING PLATES AND FALSEWORK SHALL BE SUBMITTED TO THE ENGINEER IN SUFFICIENT TIME TO PERMIT CAREFUL CHECKING PRIOR TO FABRICATION.
- INSPECTION OF WELDS INCLUDING RADIOGRAPHIC TESTING (RT) AND MAGNETIC PARTICLE TESTING (MT) SHALL BE IN ACCORDANCE WITH THE RI STANDARD SPECIFICATIONS AND THE AASHTO/AWS BRIDGE WELDING CODE, EXCEPT THAT THE REMAINING PERCENTAGE OF ALL GROOVE WELDS NOT RT TESTED SHALL BE MT.
- STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO THE LATEST PROVISIONS OF AASHTO DESIGNATION M 270 GRADE 50 AS DESIGNATED ON THE PLANS. UNLESS OTHERWISE NOTED ALL STRUCTURAL STEEL SHALL BE GRADE 50.
- ALL AASHTO M 270 STRUCTURAL STEEL USED IN GIRDERS (INCLUDING CONNECTION PLATES AND STIFFENERS), SHALL MEET THE ZONE 2 CHARPY V-NOTCH FRACTURE TOUGHNESS TEST REQUIREMENTS AS SPECIFIED IN TABLE 6.6.2-2 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR "NONFRACTURE-CRITICAL" COMPONENTS. THE ZONE 2 FRACTURE TOUGHNESS REQUIREMENTS ARE AS FOLLOWS:

NONFRACTURE-CRITICAL

GRADE 36	15 FT-LBS @ 40°F (UP TO 4 INCHES THICK)
GRADE 50 OR 50W	15 FT-LBS @ 40°F (UP TO AND INCLUDING 2 INCHES THICK)
GRADE 50 OR 50W	20 FT-LBS @ 40°F (FROM 2 INCHES THICK UP TO AND INCLUDING 4 INCHES THICK)
GRADE HPS 70W	25 FT-LBS @ -10°F (UP TO 4 INCHES THICK)

FRACTURE-CRITICAL

GRADE 36	25 FT-LBS @ 40°F (UP TO 4 INCHES THICK)
GRADE 50 OR 50W	25 FT-LBS @ 40°F (UP TO AND INCLUDING 2 INCHES THICK)
GRADE 50 OR 50W	30 FT-LBS @ 40°F (FROM 2 INCHES THICK UP TO AND INCLUDING 4 INCHES THICK)

SAMPLING AND TESTING PROCEDURES SHALL BE IN ACCORDANCE WITH AASHTO T243. THE FREQUENCY OF TESTING SHALL BE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE CHARPY V-NOTCH FRACTURE TOUGHNESS TEST REQUIREMENT IS NOT MANDATORY FOR THE FOLLOWING STEEL COMPONENTS:

- SUPPORT AND EXCAVATION COMPONENTS

- WELDING SHALL BE IN ACCORDANCE WITH THE LATEST STRUCTURAL WELDING CODE AASHTO/AWS D1.5 INCLUDING ALL INTERIMS TO DATE) AND APPLICABLE SUPPLEMENTAL AWS PUBLICATIONS.
- ALL HIGH STRENGTH BOLTS SHALL CONFORM TO AASHTO DESIGNATION M164 (ASTM A325, TYPE 1), AND SHALL BE MECHANICALLY GALVANIZED IN ACCORDANCE WITH AASHTO M298 (ASTM B695) CLASS 50. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH SECTION 824 OF THE RI STANDARD SPECIFICATIONS. ALL NUTS AND WASHERS SHALL BE MECHANICALLY GALVANIZED IN ACCORDANCE WITH M05.04.4 OF THE RI STANDARD SPECIFICATIONS. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL CONNECTIONS ARE "SLIP CRITICAL" WITH CLASS B SURFACE CONDITIONS.
- WASHERS MEETING AASHTO DESIGNATION M 293 ARE TO BE USED OVER ALL HOLES THAT ARE MORE THAN $\frac{1}{16}$ " IN DIAMETER GREATER THAN THE BOLT DIAMETER AND UNDER ALL PARTS TURNED DURING ASSEMBLY.
- WELDING ELECTRODES SHALL HAVE THE SAME CORROSION RESISTANCE AS THE BASE METAL AND SHALL BE FREE OF MOISTURE AT THE TIME OF USE.
- UNLESS OTHERWISE SPECIFIED, STRUCTURAL STEEL SHALL BE PREPARED AND PAINTED IN ACCORDANCE WITH THE RI STANDARD SPECIFICATIONS.

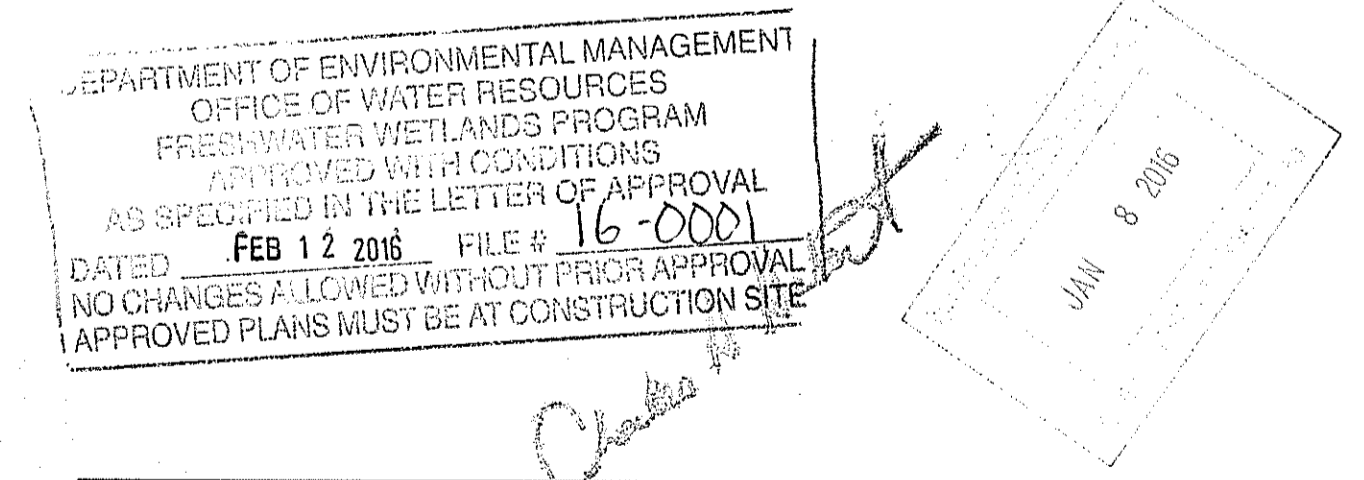
STRUCTURAL STEEL NOTES (CONTINUED)

- PRIOR TO FABRICATION, ALL MATERIALS SHALL BE BLAST-CLEANED TO AT LEAST SSPC-SP6 TO REMOVE ALL OIL, DIRT, GREASE, MILL SCALE AND OTHER DELETERIOUS MATERIALS FROM THE SURFACES OF THE STEEL TO BE FABRICATED.
- PRIOR TO SHOP COATING AS SPECIFIED IN SECTION 825 OF THE RI STANDARD SPECIFICATIONS, ALL CORNERS AND EDGES OF STEEL WHICH HAVE BEEN FLAME CUT OR OTHERWISE HARDENED SHALL BE SOFTENED BY GRINDING OR BLAST-CLEANING TO PROVIDE A SURFACE SUITABLE FOR APPLICATION OF THE SPECIFIED PAINT SYSTEM.
- WELDING OF ATTACHMENTS TO GIRDER FLANGES OR WEBS FOR CONSTRUCTION PURPOSES IS NOT PERMITTED EXCEPT WHEN APPROVED BY THE ENGINEER.
- THE ENDS OF ALL BEAMS SHALL BE VERTICAL AFTER ALL DEAD LOADS HAVE BEEN PLACED.
- INTERMEDIATE CONNECTION PLATES SHALL BE PLACED ON THE INTERIOR SIDE OF THE FASCIA BEAM WEBS AND ON BOTH SIDES OF ALL INTERIOR BEAM WEBS.
- BEARING STIFFENERS SHALL BE FABRICATED AS SHOWN ON THE PLANS AND SHALL BE PLACED ON BOTH SIDES OF ALL BEAM WEBS.
- INTERMEDIATE STIFFENERS AND CONNECTION PLATES SHALL BE SET PERPENDICULAR TO THE FLANGES OF THE BEAMS.
- END BEARING STIFFENERS AT BEAM ENDS SHALL BE PLUMB.
- BOLTED CONNECTIONS SHALL BE DESIGNED AS SLIP-CRITICAL CONNECTIONS. THE FAYING SURFACES SHALL SATISFY CLASS B SURFACE CONDITION AS DEFINED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THE BEAMS SHALL BE CAMBERED TO THE AMOUNTS SHOWN ON THE PLANS. THE FABRICATOR'S SHOP DRAWINGS SHALL INCLUDE, IN ADDITION TO ANY CUTTING OR CAMBER DIAGRAMS NECESSARY FOR THEIR PURPOSES, A SHOP ASSEMBLY DIAGRAM WHICH PROVIDES CAMBER OFFSETS CALCULATED BY THE FABRICATOR AT THE REFERENCE POINTS PROVIDED BY THE ENGINEER (USUALLY TENTH POINTS OF THE SPAN). THE INFORMATION PROVIDED SHALL BE SUFFICIENT ENOUGH FOR THE ENGINEER TO EVALUATE WHETHER THE CAMBER HAS BEEN CORRECTLY INTERPRETED. AT THE TIME AND PLACE OF ERECTION, BEAMS SHALL HAVE THE REQUIRED AMOUNT OF CAMBER. THE ERECTED VERTICAL ALIGNMENT (CAMBER) SHALL NOT DEVIATE FROM THE THEORETICAL ERECTED VERTICAL ALIGNMENT BY MORE THAN FOLLOWING: $-0, +\frac{1}{4}$ " x (TOTAL LENGTH, IN FEET, FROM THE NEAREST SUPPORT)/10. THE MAXIMUM DEVIATION IS $1\frac{1}{2}$ " BETWEEN SUPPORTS.
- ALL SHOP CONNECTIONS AND SPLICES SHALL BE WELDED. WELDING PROCEDURES AND TECHNIQUES TO BE USED IN FABRICATION AND ERECTION OF THE BEAMS SHALL BE AS SHOWN ON THE SHOP DRAWINGS AND SHALL INCORPORATE THE FOLLOWING:
 - ALL WEB AND FLANGE SPLICES OTHER THAN THOSE SHOWN ON THE PLANS MUST BE APPROVED BY THE ENGINEER. ALTERNATE OR ADDITIONAL SPLICES ARE TO BE LOCATED AND DESIGNED BY THE FABRICATOR AND SHOWN ON THE SHOP DRAWINGS. THESE SPLICES ARE TO FULLY DEVELOP THE STRENGTH OF THE WEB AND FLANGE PLATES. WEB SPLICES, IF USED, SHALL BE LOCATED 2'-0" MINIMUM FROM ANY STIFFENER.
 - NO MORE THAN TWO SHOP WEB SPLICES WILL BE PERMITTED BETWEEN FIELD SPLICES. SPLICING OF BEAMS BY FIELD WELDING WILL NOT BE PERMITTED.
- NO SHOP FILLET WELD SHALL BE LESS THAN $\frac{1}{4}$ ".
- WHEN STEEL DIE STAMPS ARE USED TO IDENTIFY PIECES AND MEMBERS, FABRICATORS SHALL UTILIZE LOW STRESS STAMPS.
- FOR SIZE AND LOCATION OF ANCHOR BOLTS, SEE BEAM SEAT DRAWINGS.
- THE DESIGN OF THE STRUCTURE IS BASED ON THE SELF-WEIGHT OF THE STRUCTURAL STEEL IN ITS COMPLETELY ERECTED CONFIGURATION ONLY. DEFLECTION INCURRED DURING THE VARIOUS STAGES OF THE ERECTION PROCESS ARE NOT CONSIDERED. THEREFORE, THE ACTUAL ERECTION METHODS AND SEQUENCES EMPLOYED BY THE CONTRACTOR MAY HAVE A SUBSTANTIAL EFFECT ON (1) THE TOTAL STRESS, I.E. THE DESIGN PLUS ERECTION STRESS, AND/OR (2) THE STEEL PROFILE AS ERECTED. THE CONTRACTOR SHALL SUBMIT AN ERECTION PROCEDURE IN ACCORDANCE WITH THE PROVISIONS OF SECTION 824.03.10 ERECTION, OF THE STANDARD SPECIFICATIONS. THE CONTRACTOR SHALL PROVIDE CALCULATIONS FOR ALL PHASES AND ERECTION CONDITIONS WHICH DEMONSTRATE THAT THE ALLOWABLE STRESSES ARE NOT EXCEEDED AND THAT THE GEOMETRY AS ERECTED (HORIZONTAL AND VERTICAL) WILL BE CONSISTENT WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. ANY CORRECTIVE WORK NECESSARY TO REPOSITION PREVIOUSLY ERECTED STEEL TO ACHIEVE ACCEPTABLE ALIGNMENT AND PROFILE MUST BE APPROVED BY THE ENGINEER AND SHALL BE PERFORMED AT NO EXTRA COST TO THE STATE.
- IF THE BEAMS CANNOT BE SHIPPED IN THE LENGTHS SHOWN ON THE PLANS, FIELD SPlice(S) WILL BE PERMITTED AT THE REQUEST OF THE CONTRACTOR, BUT NO COMPENSATION WILL BE ALLOWED FOR THE SPLICES.
- IF THE BEAMS CAN BE FABRICATED IN LENGTHS LONGER THAN THE SECTIONS SHOWN ON THE PLANS BY ELIMINATING THE FIELD SPLICES, FIELD SPLICES MAY BE OMITTED AT THE REQUEST OF THE CONTRACTOR. THE CONTRACTOR ASSUMES FULL RESPONSIBILITY FOR SECURING A HAULING PERMIT. APPROVAL FOR ELIMINATION OF A FIELD SPlice AT THE SHOP DRAWING STAGE DOES OBLIGATE THE DEPARTMENT TO ISSUE A HAULING PERMIT.

SHOP DRAWING SUBMITTAL

- CRANE SUBMITTALS
- CONCRETE: MIX DESIGNS, PLACING AND METHODS AND EQUIPMENT, CURING PLAN AND METHODS, PERSONNEL RESOURCES
- REINFORCING STEEL, SPLICES AND INSERTS
- BRIDGE STATE SEAL TABLETS
- STRUCTURAL STEEL
- CONCRETE SUBCONTRACTOR'S QUALIFICATIONS AND EXPERIENCE
- STRUCTURAL COMPUTATIONS
- BRIDGE DEMOLITION; EQUIPMENT, DETAILED SEQUENCE OF WORK
- EARTH SUPPORT SYSTEMS/COFFERDAMS (SHEETING, ETC.)
- TEMPORARY PROTECTION SHIELDS FOR DEMOLITION AND CONSTRUCTION
- ARCHITECTURAL TREATMENTS (SPECIAL FORMS/LINERS, GRANITE VENEER, ETC.)
- ERECTION PROCEDURES; EQUIPMENT (TYPE/SIZE AND PLACEMENT), DETAILED SEQUENCE OF WORK
- BRIDGE RAILINGS
- BRIDGE ID TABLETS
- PRE CONSTRUCTION SURVEY, PRE/POST CONSTRUCTION SUMMARY OF SURROUNDING WATERWAY (BY GEOTECHNICAL DESIGN FIRM)
- PAINTING
- WELDING PROCEDURES; WELD SPLICES
- GRS/IBS
- TIMBER

NECESSARY SUBMITTALS MAY NOT BE LIMITED TO THE ABOVE LIST AND MAY REQUIRE OTHER SUBMITTALS AT THE RESIDENT ENGINEER'S REQUEST FOR; SHOP DRAWINGS, CERTIFICATE OF COMPLIANCE, PRODUCT INFORMATION, CATALOG CUTS, TEST DATA OR OTHER.



REVISIONS			RHODE ISLAND DEPARTMENT OF TRANSPORTATION DESIGN/BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE EXETER, RHODE ISLAND JOB SPECIFIC GENERAL NOTES 3
NO.	DATE	BY	

GENERAL NOTES REGARDING TEMPORARY CONSTRUCTION CONDITIONS

1. DESIGN WIND PRESSURES FOR CONSTRUCTION:

MINIMUM WIND PRESSURES TO BE USED BY THE CONTRACTOR FOR DESIGN DURING THE CONSTRUCTION CONTRACT (WITH THE EXCEPTION OF SIGNS) SHALL BE FROM THE FOLLOWING TABLE:

HEIGHT ABOVE GROUND (FEET)	WIND PRESSURE (PSF)
UP TO 17'	23
OVER 17' AND UP TO 33'	27
OVER 33' AND UP TO 50'	30
OVER 50' AND UP TO 75'	34
OVER 75' AND UP TO 100'	37

TABLE NOTES

APPLICATION OF THE TABULAR PRESSURE:

- BRIDGE COMPONENTS DURING CONSTRUCTION, PRIOR TO THE INSTALLATION OF THE PERMANENT BRACING SYSTEMS, NOT INCLUDING CRANE LIFTING.
- FALSE WORK, SHORING, AND SCAFFOLDING AS DEFINED IN FHWA "GUIDE DESIGN SPECIFICATION FOR BRIDGE TEMPORARY WORKS", EXCLUDING 3-DIMENSIONAL LATTICED OR TRUSSED FRAMES OR TOWERS;
- TEMPORARY SHIELDING.

WIND PRESSURES FOR ALL OTHER STRUCTURES SHALL BE CALCULATED BASED ON ASCE "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION", SEI/ASCE 37-02 (ALL REFERENCES TO THE ASCE 7 IN THE SEI/ASCE 37-02 PUBLICATION, SHALL BE THE LATEST REVISION OF ASCE 7). THE EXPOSURE CATEGORY SHALL BE B.

2. ERECTION OF BRIDGE COMPONENTS:

FOR THE ERECTION OF STRUCTURES, THE FOLLOWING SHALL APPLY:

- THE CONTRACTOR SHALL SUBMIT AN ERECTION PLAN THAT PROVIDES COMPLETE DETAILS OF THE PROCESS INCLUDING, BUT NOT LIMITED TO, TEMPORARY SUPPORTS, SCHEDULING AND OPERATION SEQUENCING, CRANE PLACEMENT, AND ASSUMED LOADS AND CALCULATED STRESSES DURING VARYING STAGES OF LIFTING. THIS APPLIES TO STRUCTURES OF ANY KIND. THE CAPACITY OF THE CRANE AND ALL LIFTING AND CONNECTING DEVICES SHALL BE ADEQUATE FOR 125 PERCENT OF THE TOTAL PICK LOAD INCLUDING SPREADERS AND OTHER MATERIALS. THIS FACTOR OF SAFETY SHALL BE IN ADDITION TO ALL MANUFACTURERS' PUBLISHED FACTORS OF SAFETY.
 - A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE OF RHODE ISLAND, WILL BE REQUIRED TO STAMP THE CONTRACTOR'S ERECTION PLAN.
 - THE CONTRACTOR'S PROFESSIONAL ENGINEER WILL BE REQUIRED TO INSPECT AND PROVIDE WRITTEN APPROVAL OF EACH PHASE OF A GIRDER INSTALLATION, PRIOR TO ALLOWING VEHICLES OR PEDESTRIANS ON OR BELOW THE STRUCTURE. THE PROFESSIONAL ENGINEER MUST ALSO STAMP ALL CHANGES TO THE CONTRACTOR'S ERECTION PLAN. ADDITIONALLY, ALL PROPOSED CHANGES MUST BE SUBMITTED TO RIDOT FOR REVIEW AND APPROVAL PRIOR TO IMPLEMENTATION.
 - A MANDATORY PRE-ERECTION CONFERENCE WILL BE HELD AT LEAST TWO WEEKS PRIOR TO THE START OF THE GIRDER INSTALLATION TO DISCUSS THE PLAN AND PROCEDURES, WORK SCHEDULES, CONTINGENCY PLANS, SAFETY REQUIREMENTS AND TRAFFIC CONTROL. THE CONTRACTOR'S PROFESSIONAL ENGINEER AND ERECTION SUBCONTRACTOR WILL BE REQUIRED TO ATTEND THIS MEETING, AS WILL THE RIDOT RESIDENT ENGINEER, THE DESIGN PROJECT ENGINEER AND THE DESIGN CONSULTANT. BASED UPON DISCUSSIONS AT THIS MEETING AND A REVIEW OF THE CONTRACTOR'S ERECTION PLAN, RIDOT MAY ORDER THE CONTRACTOR TO MODIFY AND RESUBMIT THE ERECTION PLAN TO THE ENGINEER FOR REVIEW AND APPROVAL.
 - THE CONTRACTOR WILL BE REQUIRED TO PERFORM DAILY INSPECTIONS OF THE ERECTED GIRDERS UNTIL THE BRIDGE DECK IS COMPLETELY POURED.
 - THE COST OF PREPARING AND STAMPING THE ERECTION PLAN, COMPUTATIONS, AND REPORTS, RESPONDING TO RIDOT'S COMMENTS AND MAKING THE NECESSARY REVISIONS, AND ATTENDANCE AT MEETINGS SHALL BE CONSIDERED INCIDENTAL TO THE COST OF THE SUPERSTRUCTURE PAY ITEM, BE IT CONCRETE, STEEL OR TIMBER.
3. ANY PERMANENT AND/OR TEMPORARY CONDITIONS INCLUDED ON THE ERECTION PLAN WHICH ARE NOT INCLUDED ON THE RIDEM APPROVED PLANS ARE SUBJECT TO WRITTEN APPROVAL BY RIDEM PRIOR TO APPROVAL BY RIDOT. SUCH APPROVAL SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE ACCOMPLISHED AT NO COST TO THE STATE. ALL SUBMISSIONS OF ADDITIONAL INFORMATION TO RIDEM MUST BE COORDINATED THROUGH THE RIDOT NATURAL RESOURCES UNIT.

CONSTRUCTION NOTES

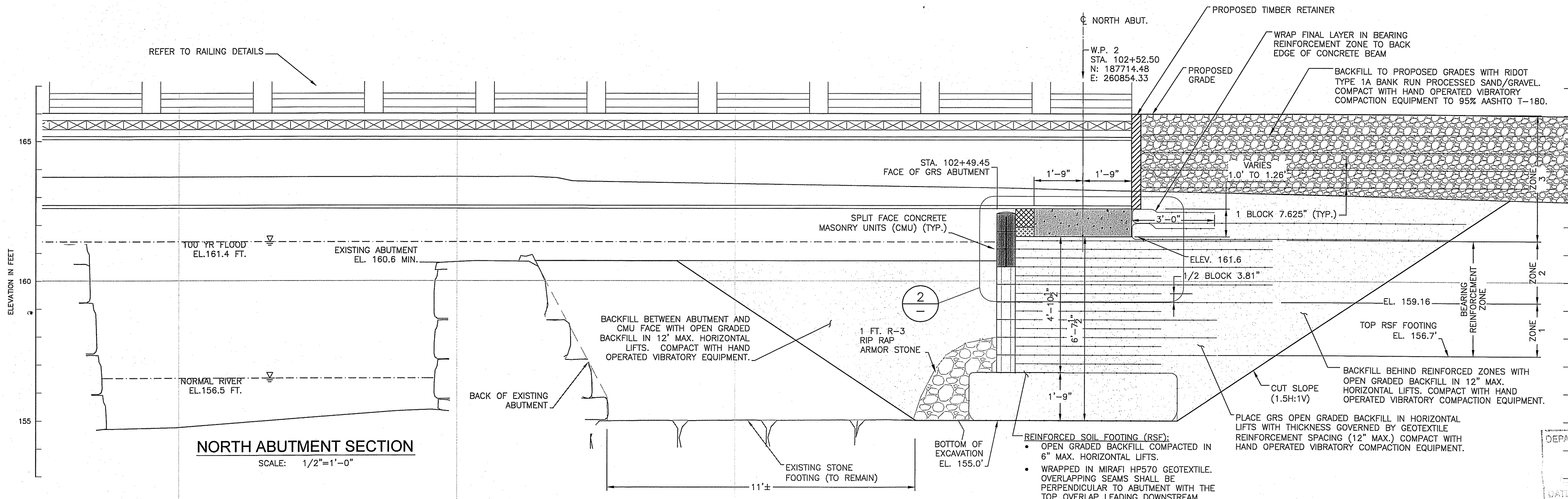
- THE CONTRACTOR IS RESPONSIBLE FOR THE IMPLEMENTATION, CONSTRUCTION, OPERATION AND SAFETY OF ALL EQUIPMENT AND PROCEDURES.
- THE CONTRACTOR SHALL SUBMIT WORKING DOCUMENTS SHOWING PROPOSED METHODS OF LIFTING, SEQUENCING OF LIFTING, LOCATION OF CRANES, CRANE CAPACITIES, LOCATION OF THE LIFTING POINTS ON THE BRIDGE COMPONENTS, WEIGHTS OF THE COMPONENTS, LIFTING DEVICES AND LOAD DISTRIBUTION DEVICE DETAIL. THE METHOD AND ALL SUBMISSIONS SHALL BE PREPARED AND STAMPED BY A RHODE ISLAND REGISTERED PROFESSIONAL ENGINEER.
- COORDINATE ALL CONSTRUCTION ACTIVITIES WITHIN THE WORKING AREA WITH RIDOT REGARDING UTILITIES, PROTECTION OF TRAFFIC AND SCHEDULE.
- THE CONTRACTOR SHALL EXERCISE EXTREME CARE TO AVOID DAMAGE TO EXISTING STRUCTURES. ALL STRUCTURES DAMAGED AS A RESULT OF THE CONTRACTOR'S OPERATION SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER AT NO ADDITIONAL COST TO THE STATE.
- ALL SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER IN SUFFICIENT TIME TO PERMIT CAREFUL CHECKING AS NOT TO DELAY THE PROJECT.
- ALL RIGGING IS TO BE IN EXCELLENT WORKING CONDITION.
- UNLOADED CRANES ARE ALLOWED TO TRAVEL IN THE WORKING AREA.
- CRANE DELIVERY LOCATIONS MAY VARY AS LONG AS MAXIMUM CRANE RADIUS IS NOT EXCEEDED.
- THE CONTRACTOR SHALL NOT BE PERMITTED TO ANCHOR HEAVY EQUIPMENT TO THE EXISTING BRIDGE DURING CONSTRUCTION.
- TEMPORARY EXCAVATION SUPPORT SYSTEM SHALL BE DESIGNED, FURNISHED AND INSTALLED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO THE START OF ANY EXCAVATION.
- CONTRACTOR SHALL SECURE ALL WORK AREAS AT ALL TIMES TO PREVENT UNAUTHORIZED ACCESS.
- STOCKPILED SOIL TO BE 30 FEET FROM PIERS, WALLS AND ABUTMENTS.
- THE CONTRACTOR SHALL NOT BE ALLOWED TO USE RIVER WATER DURING CONSTRUCTION. IN ADDITION, DISCHARGE FROM THE CONSTRUCTION ACTIVITIES SHALL NOT BE PERMITTED.

DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
 OFFICE OF WATER RESOURCES
 PERMITTED WETLANDS PROGRAM
 AS SHOWN IN THE LETTER OF APPROVAL
 FEB 12 2016 FILE # 16-0001
 NO CHANGES ALLOWED WITHOUT PRIOR APPROVAL
 APPROVED PLANS MUST BE AT CONSTRUCTION SITE

REVISIONS			RHODE ISLAND DEPARTMENT OF TRANSPORTATION
NO.	DATE	BY	
			DESIGN/BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE EXETER, RHODE ISLAND
			JOB SPECIFIC GENERAL NOTES 4
CHECKED BY _____			DATE 11/4/15 SCALE NO SCALE

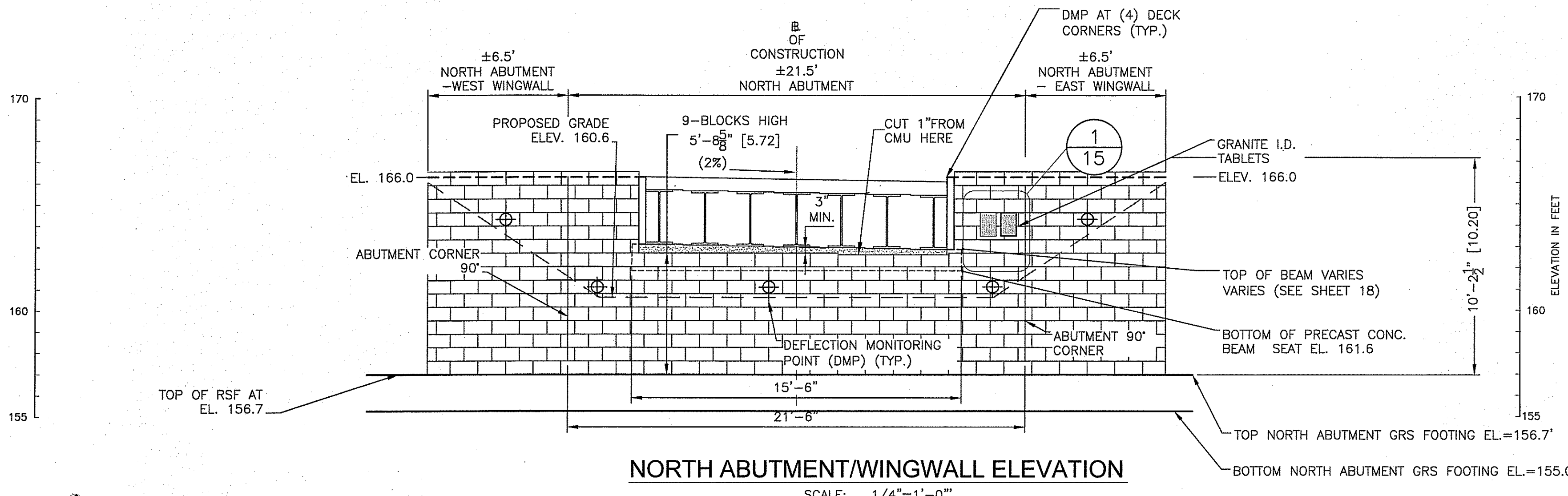


FED. ROAD DIV. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
RI	BRO-0784(002)	2015	13	32	

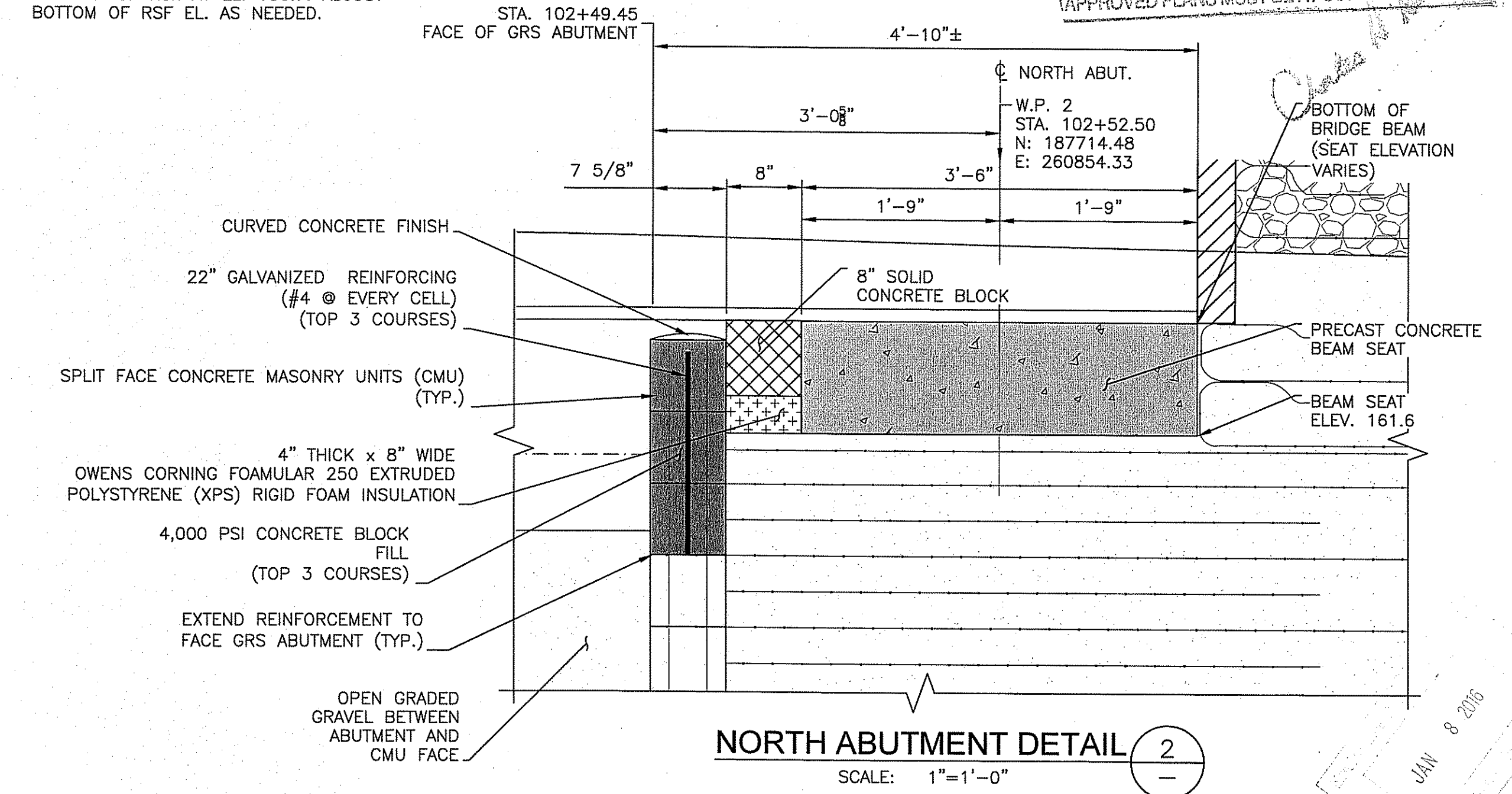


NORTH ABUTMENT SECTION
SCALE: 1/2"=1'-0"

DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
FEB 12 2016
NO CHANGES ALLOWED WITHOUT PRIOR APPROVAL
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NORTH ABUTMENT/WINGWALL ELEVATION
SCALE: 1/4"=1'-0"



NORTH ABUTMENT DETAIL
SCALE: 1"=1'-0"

REINFORCEMENT SCHEDULE - NORTH ABUTMENT			
ZONE	TYPE	LENGTH	VERTICAL SPACING
1	MIRAFI HP570	8 FT (NOTE 1)	1 BLOCK
2	MIRAFI HP570	10 FT (NOTE 1)	1 BLOCK
3	MIRAFI HP570	VARIES (NOTE 2)	1 BLOCK
BEARING REINFORCEMENT	MIRAFI HP570	6 FT (NOTE 3)	1/2 BLOCK

- NOTES:**
- DISTANCE FROM FRONT FACE OF GRS FACING BLOCKS
 - DISTANCE FROM END OF BRIDGE BEAM TO CUT SLOPE
 - DISTANCE FROM INSIDE FACE OF CMU BLOCKS

- NOTES**
- BASE MAP DEVELOPED FROM AN AUTOCAD DRAWING FILE, "72528EX.DWG", PREPARED BY VANASSE HANGEN BRUSTLIN AND TRANSMITTED TO GZA ON MAY 17, 2013.

REVISIONS		
NO.	DATE	BY

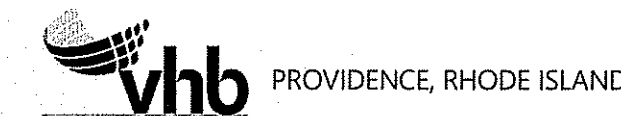
**RHODE ISLAND
DEPARTMENT OF TRANSPORTATION**

**DESIGN/BUILD SERVICES FOR
REPLACEMENT OF
FALLS RIVER BRIDGE**

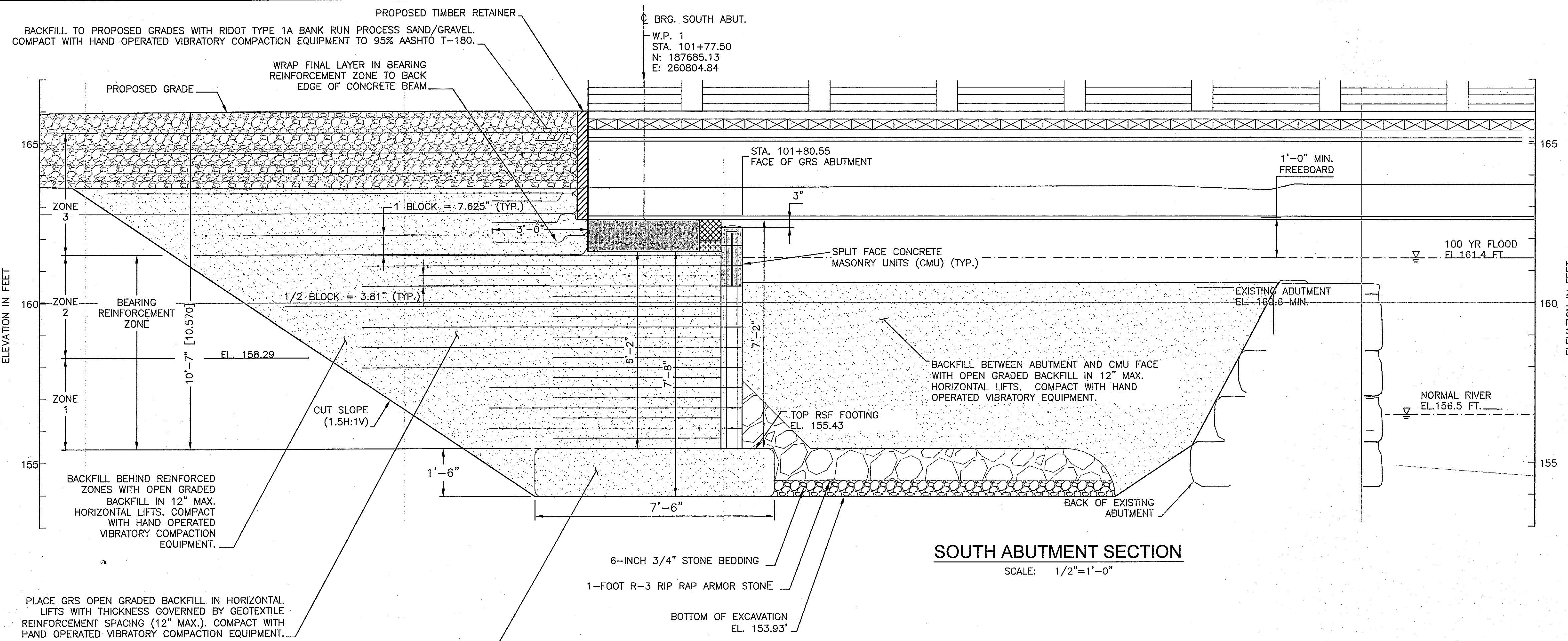
EXETER, RHODE ISLAND

**NORTH ABUTMENT
SECTIONS AND DETAILS**

CHECKED BY _____ DATE 11/4/15 SCALE AS SHOWN



FED. ROAD DIV. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	RI	BR0-0784(002)	2015	14	32

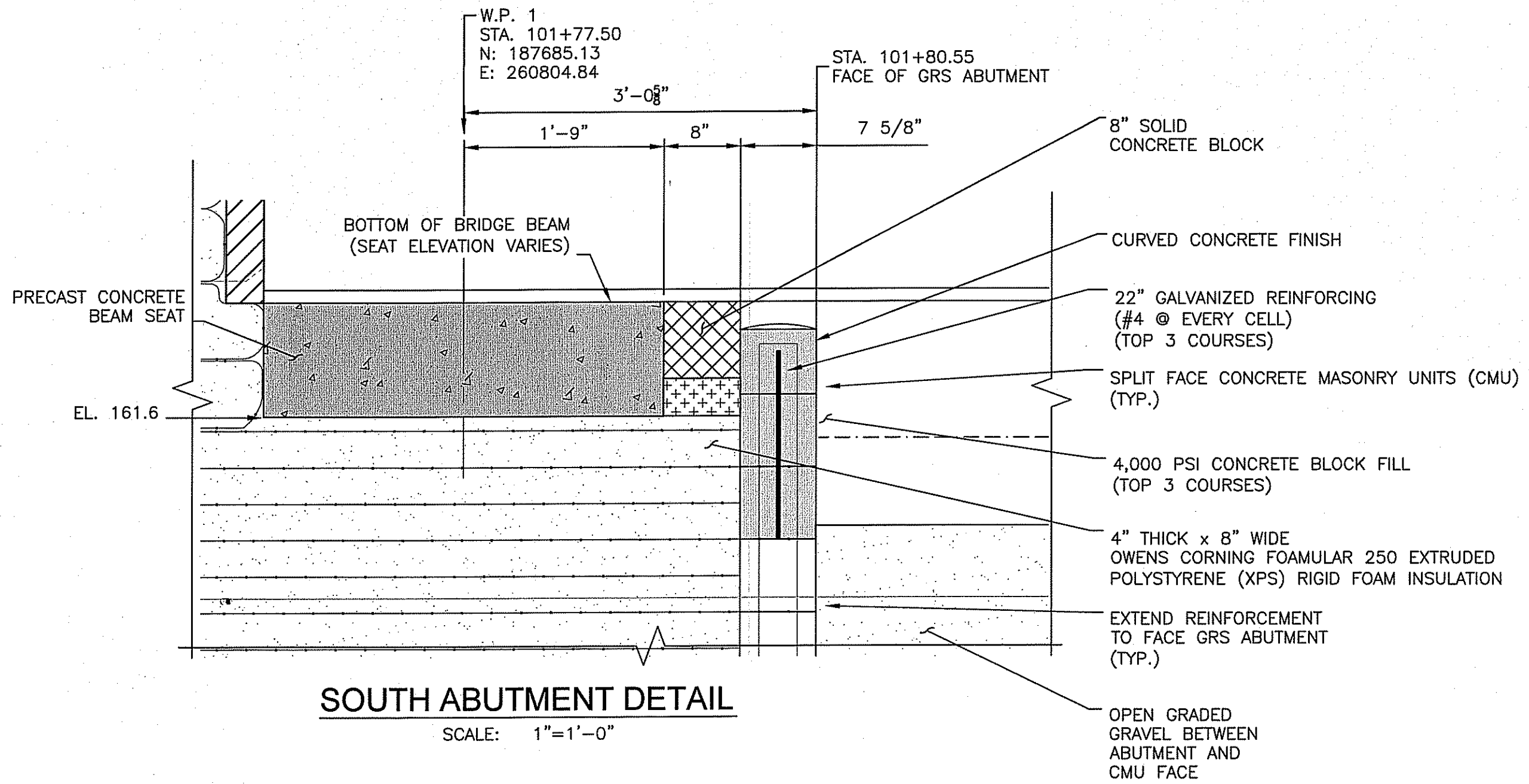


SOUTH ABUTMENT SECTION
SCALE: 1/2"=1'-0"

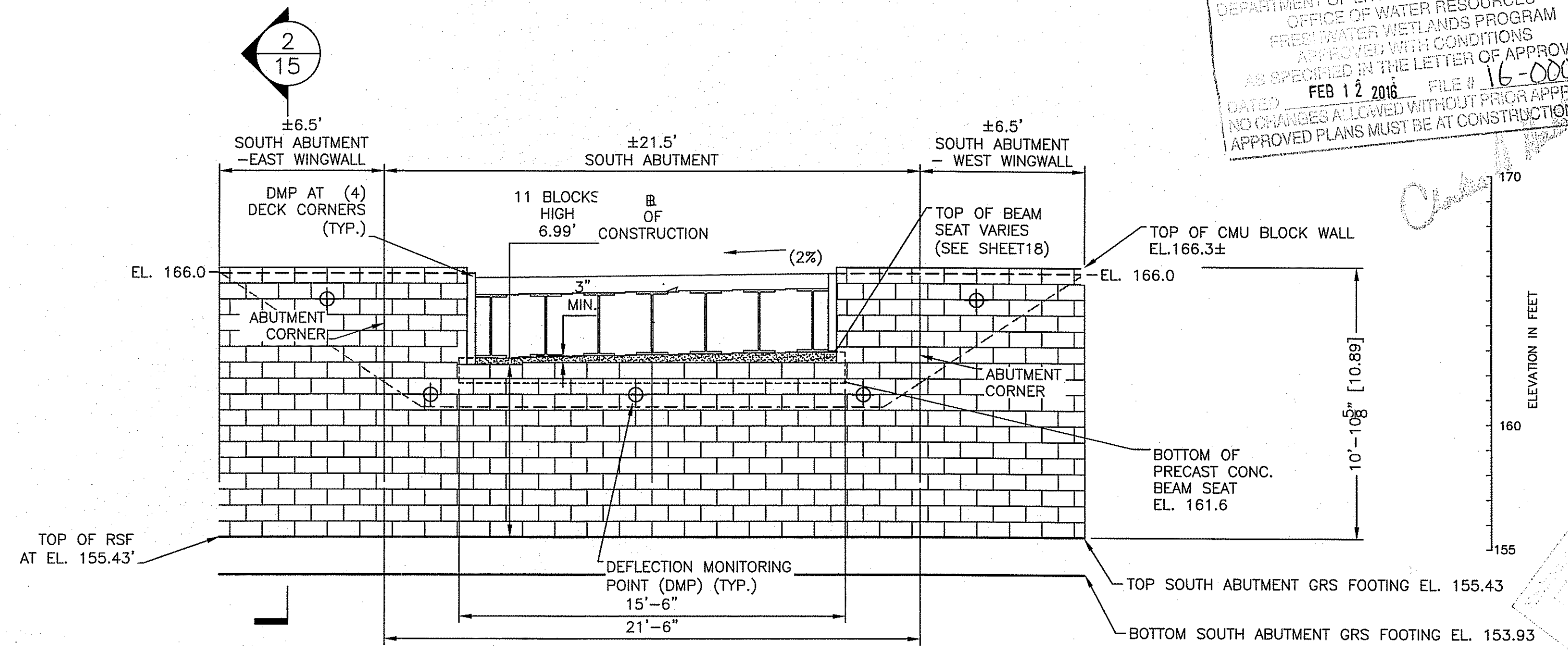
REINFORCEMENT SCHEDULE - SOUTH ABUTMENT			
ZONE	TYPE	LENGTH	VERTICAL SPACING
1	MIRAFI HP570	8 FT (NOTE 1)	1 BLOCK
2	MIRAFI HP570	12 FT (NOTE 1)	1 BLOCK
3	MIRAFI HP570	VARIABLES (NOTE 2)	1 BLOCK
BEARING REINFORCEMENT	MIRAFI HP570	6 FT (NOTE 3)	1/2 BLOCK

- NOTES:
- DISTANCE FROM FRONT FACE OF GRS FACING BLOCKS
 - DISTANCE FROM END OF BRIDGE BEAM TO CUT SLOPE
 - DISTANCE FROM INSIDE FACE OF CMU FACING BLOCKS

- REINFORCED SOIL FOOTING (RSF):**
- OPEN GRADED BACKFILL COMPACTED IN 6" MAX. HORIZONTAL LIFTS.
 - WRAPPED IN MIRAFI HP570 GEOTEXTILE. OVERLAPPING SEAMS SHALL BE PERPENDICULAR TO ABUTMENT WITH THE TOP OVERLAP LEADING DOWNSTREAM.



SOUTH ABUTMENT DETAIL
SCALE: 1"=1'-0"



SOUTH ABUTMENT/WINGWALL ELEVATION
SCALE: 1/4"=1'-0"

DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
OFFICE OF WATER RESOURCES
FRESH WATER WETLANDS PROGRAM
APPROVED WITH CONDITIONS
AS PROVIDED IN THE LETTER OF APPROVAL
DATED FEB 12 2016 FILE # 16-0001
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NOTES

- BASE MAP DEVELOPED FROM AN AUTOCAD DRAWING FILE, "72528EX.DWG", PREPARED BY VANASSE HANGEN BRUSTLIN AND TRANSMITTED TO GZA ON MAY 17, 2013.

REVISIONS		
NO.	DATE	BY

**RHODE ISLAND
DEPARTMENT OF TRANSPORTATION**

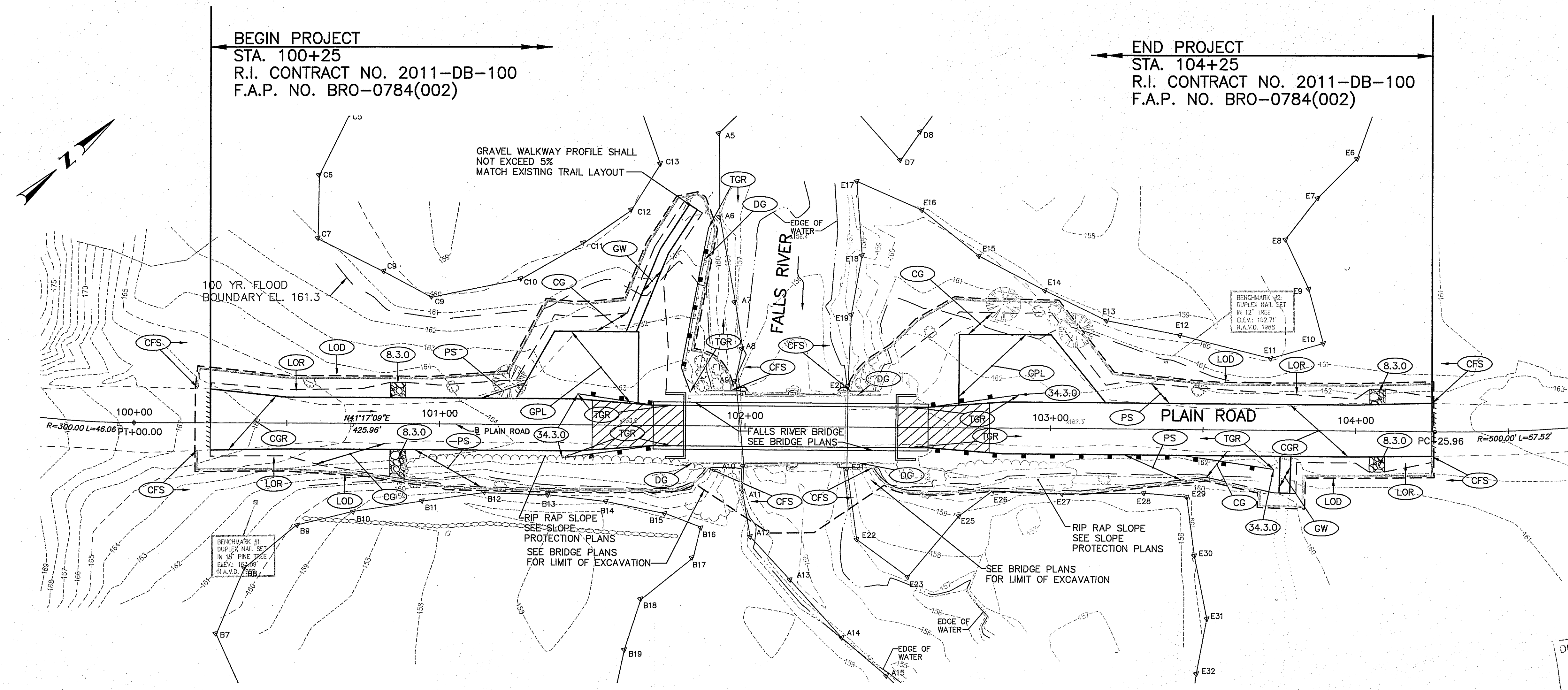
**DESIGN/BUILD SERVICES FOR
REPLACEMENT OF
FALLS RIVER BRIDGE**

EXETER, RHODE ISLAND

**SOUTH ABUTMENT
SECTIONS AND DETAILS**

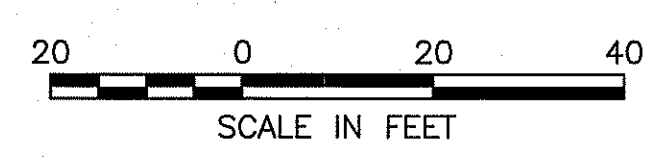
CHECKED BY _____ DATE 11/4/15 SCALE AS SHOWN





BEGIN PROJECT
 STA. 100+25
 R.I. CONTRACT NO. 2011-DB-100
 F.A.P. NO. BRO-0784(002)

END PROJECT
 STA. 104+25
 R.I. CONTRACT NO. 2011-DB-100
 F.A.P. NO. BRO-0784(002)



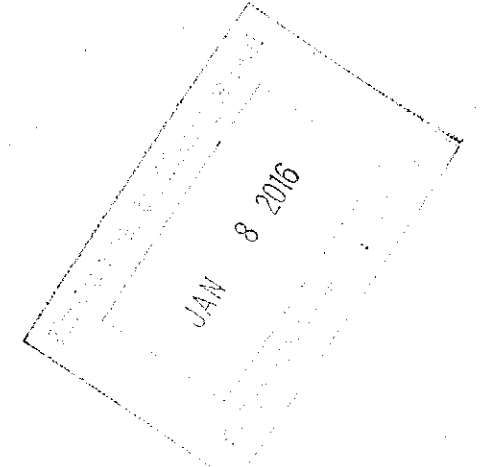
TREE REMOVAL/TREE TRIMMING			
STATION	OFFSET	TYPE	REMOVE/LIMB
100+60	L 16 FT	30" PINE	LIMB
100+95	R 18 FT	24" OAK	LIMB
101+05	L 16 FT	30" PINE	LIMB
101+20	R 17 FT	14" MAPLE	LIMB
101+20 TO 101+35	L 12 FT TO 16 FT	BRUSH	REMOVE
100+90 TO 101+60	R 11 FT TO 16 FT	BRUSH	REMOVE
101+60 TO 101+95	R 11 FT TO 24 FT	BRUSH W/SMALL TREES	REMOVE
101+60 TO 101+95	L 11 FT TO 24 FT	SMALL TREES <4" DIA.	REMOVE
102+35	R 17 FT	SMALL TREES <4" DIA.	REMOVE
102+62	R 16 FT	24" OAK	REMOVE
103+35 TO 102+65	L 11 FT TO 24 FT	SMALL TREES <4" DIA.	REMOVE
102+65	L 29 FT	2-24" OAK	LIMB
102+75 TO 103+50	R 12 FT TO 16 FT	BRUSH	REMOVE
102+90	L 40 FT	2-24" PINE	LIMB
102+90 TO 103+15	L 40 FT TO 20 FT	BRUSH	REMOVE
103+25	R 15 FT	2-12" MAPLE	REMOVE
103+45	L 15 FT	12" OAK	LIMB

- NOTES:**
- SEE RAILING DETAILS FOR GUARDRAIL POST SPACING AND TERMINALS.
 - FLEXIBLE DELINEATOR POSTS SHALL BE PLACED 5' O.C. ALONG BACK OF GUARDRAIL IN PARKING AREAS.
 - CONTRARY TO DRAINAGE & EROSION CONTROL NOTE 3 ON PLAN SHEET STANDARD NOTES-1, PREVIOUSLY DISTURBED GRAVEL AREAS MAY BE USED FOR MATERIAL STOCKPILE LOCATIONS DUE TO THE LIMITED SPACE AVAILABLE NEAR THE WORK SITE. STOCKPILES WILL BE RINGED WITH COMPOST FILTER SOCK TO PREVENT MIGRATION OF MATERIALS INTO SENSITIVE AREAS.
 - THE ENTIRE WORK AREA IS WITHIN THE 200 FOOT RIVERBANK WETLAND ASSOCIATED WITH FALLS RIVER.
 - SEE CONSTRUCTION STAGING PLAN FOR STOCKPILE LOCATIONS.

= LIMITS OF CROSS SECTION TRANSITION TO NORMAL CROWN TO FULL WIDTH BRIDGE SECTION. (SEE LOCATION PLANS)

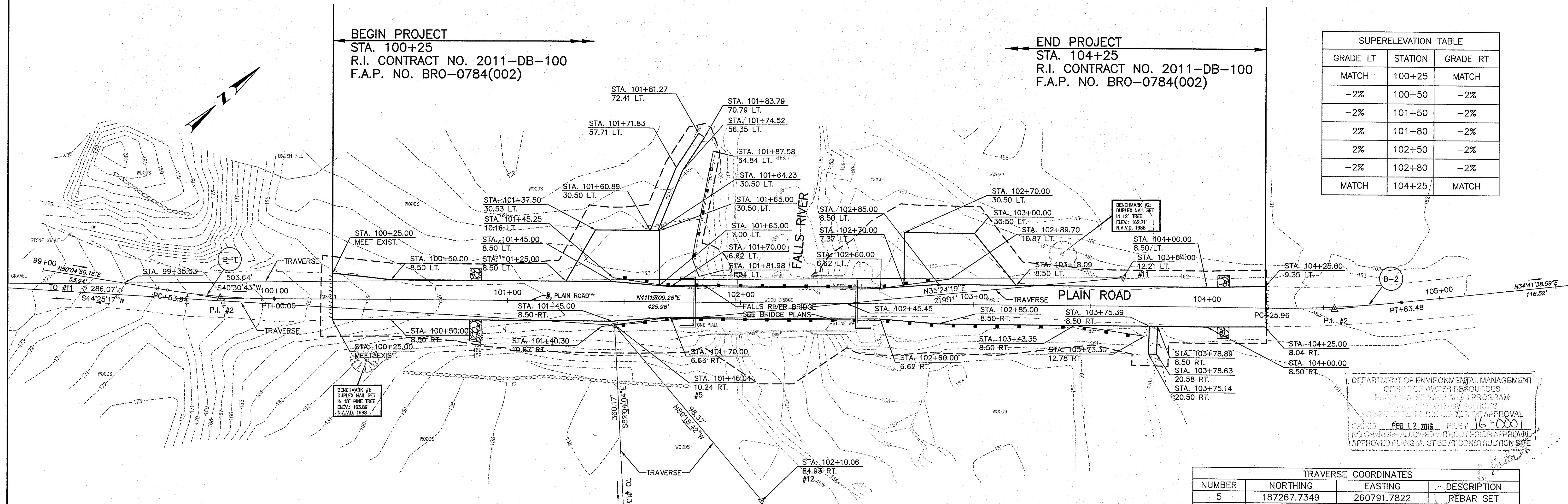
DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
 OFFICE OF WATER RESOURCES
 FRESHWATER WETLANDS PROGRAM
 APPROVED WITH CONDITIONS
 AS SPECIFIED IN THE LETTER OF APPROVAL
 DATED FEB 12 2015 FILE # 16-0001
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Chris A. [Signature]



REVISIONS		
NO.	DATE	BY
RHODE ISLAND DEPARTMENT OF TRANSPORTATION		
DESIGN/BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE		
EXETER,		RHODE ISLAND
GENERAL PLAN		
CHECKED BY _____ DATE 11/4/15 SCALE AS SHOWN		





GRADE LT	STATION	GRADE RT
MATCH	100+25	MATCH
-2%	100+50	-2%
-2%	101+50	-2%
2%	101+80	-2%
2%	102+50	-2%
-2%	102+80	-2%
MATCH	104+25	MATCH

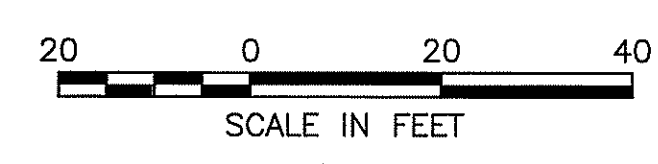
CURVE	RADIUS	LENGTH	TANGENT	DELTA	EXTERNAL
B-1	300.00	46.06	23.07	08°14'46.91"	0.89
B-2	500.00	57.52	28.79	06°35'30.67"	0.83

	PI STATION	PI NORTHING	PI EASTING	PC STATION	PC NORTHING	PC EASTING	PT STATION	PT NORTHING	PT EASTING	CC NORTHING	CC EASTING
POL	99+00.00	187457.9890	260613.4310								
B-1	99+77.02	187507.4102	260672.5008	99+53.94	187492.6038	260654.8037	100+00.00	187524.7487	260687.7255	187722.6938	260462.2976
B-2	104+54.75	187866.4621	260987.7792	104+25.96	187844.8255	260968.7803	104+83.48	187890.1368	187890.1368	188174.7339	260593.0671
POL	106+00.00	187985.9639	261070.4891								

NUMBER	NORTHING	EASTING	DESCRIPTION
5	187267.7349	260791.7822	REBAR SET
6	187406.3273	261406.3273	REBAR SET
11	187806.3097	260918.73013	REBAR SET
12	187626.5533	260890.1421	REBAR SET
13	187423.4233	260591.5561	REBAR SET

NUMBER	NORTHING	EASTING	DESCRIPTION
1	187537.5989	260733.6645	DUPLEX NAIL SET
2	187793.1968	260893.7876	DUPLEX NAIL SET

DEPARTMENT OF ENVIRONMENTAL MANAGEMENT
OFFICE OF WATER RESOURCES
FRESHWATER PLANS PROGRAM
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NO.	DATE	BY
RHODE ISLAND DEPARTMENT OF TRANSPORTATION		
DESIGN/BUILD SERVICES FOR REPLACEMENT OF FALLS RIVER BRIDGE		
EXETER, RHODE ISLAND		
LOCATION PLAN		
CHECKED BY _____ DATE 11/4/15 SCALE AS SHOWN		

